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## **ETAG 029**

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**GUIDELINE FOR EUROPEAN TECHNICAL APPROVAL  
of  
METAL INJECTION ANCHORS FOR USE IN MASONRY**

### **Annex B (informative)**

**RECOMMENDATIONS FOR TESTS TO BE  
CARRIED OUT ON CONSTRUCTION WORKS**

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# **ANNEX B (informative): RECOMMENDATIONS FOR TESTS TO BE CARRIED OUT ON CONSTRUCTION WORKS**

## **B.1. General**

This annex is valid for injection anchors with a European Technical Approval only. The approval can cover injection anchors for use in solid masonry, hollow or perforated masonry and autoclaved aerated concrete masonry.

The characteristic resistances given in the approval for use in solid masonry are valid for the base material and the bricks which have been used in the tests or larger brick sizes and larger compressive strength of the masonry unit. The characteristic resistances given in the approval for use in hollow or perforated masonry are valid only for the bricks and blocks which have been used in the tests regarding base material, size of the units, compressive strength and configuration of the voids.

In the absence of national requirements, the characteristic resistance of the injection anchor may be determined by the following so-called "job site tests", if the injection anchor has an approval with characteristic values for the same type of base material (e.g. clay, calcium silicate, lightweight aggregate or autoclaved aerated concrete) as is present on the construction works. Furthermore, job site tests for use in solid masonry are possible only if the injection anchor has an approval for use in solid masonry and job site tests for use in hollow or perforated masonry are possible only if the injection anchor has an approval for use in hollow or perforated masonry.

This characteristic resistance to be applied to an injection anchor should be determined by means of at least 15 tests carried out on the construction work with a centric tension load acting on the injection anchor. These tests may also be performed in a laboratory under equivalent conditions as used on construction work.

Execution and evaluation of the tests as well as issue of the test report and determination of the characteristic resistance should be supervised by the person responsible for execution of works on site and be carried out by a competent person.

The number and position of the injection anchors to be tested should be adapted to the relevant special conditions of the construction work in question and, for example, in the case of blind and larger areas, be increased such that reliable information about the characteristic resistance of the injection anchor embedded in the base material in question can be derived. The tests should take account of the unfavourable conditions of practical execution.

## **B.2. Assembly**

The injection anchor to be tested should be installed (e.g. preparation of drill hole, drilling tool to be used, drill bit, type of drilling hammer or rotation, thickness of fixture) and as far as spacing and edge distances are concerned be distributed in the same way as foreseen for the intended use.

Depending on the drilling tool, hard metal hammer-drill bits or hard metal percussion drill bits according to ISO 5468:2006 [10] should be used.

The cleaning process of the drill hole should follow the manufacturer's installation instruction using the corresponding tools.

## **B.3. Execution and evaluation of tests**

### **B.3.1 General**

The characteristic resistance may be determined by pull-out tests according to B.3.2 or proof-load tests according to B.3.3. The characteristic resistance  $F_{RK1}$  or  $F_{RK2}$  has to be equal to or smaller than the characteristic resistance  $F_{RK}$  which is given in the approval for the same category of masonry (bricks or blocks).

The test rig used for the tests should allow a continuous slow increase of load recorded by calibrated measuring equipment.

The load should act perpendicular to the surface of the base material and be transmitted to the injection anchor via a hinge. The reaction forces should be transmitted to the base material such that possible breakout of the masonry is not restricted. This condition is considered as fulfilled if the support reaction forces are transmitted either in adjacent masonry units or at a distance of at least 150 mm from the injection anchors.

In absence of national regulations the partial safety factors for the resistance of the injection anchor may be taken as  $\gamma_M = 2,5$  for use in masonry.

### B.3.2 Pull-out tests

#### B.3.2.1 Execution of pull-out tests

The load should be progressively increased so that the ultimate load is achieved after not less than about 1 minute. Recording of load is carried out when the ultimate load is achieved.

#### B.3.2.2 Evaluation of the results of the pull-out tests

If the number of pull-out tests is equal to or more than 15, the characteristic resistance  $N_{Rk1}$  is obtained from the measured values of  $N_1$  as follows:

$$N_{Rk1} = 0,5 \cdot N_1 \leq N_{Rk,ETA} \quad (B.3.1.a)$$

with:  $N_1$  = mean value of the five smallest measured values at the ultimate load  
 $N_{Rk,ETA}$  = characteristic resistance  $N_{Rk}$  given in the ETA for the same category of masonry

If the number of pull-out tests is smaller than 15, the characteristic values are to be determined as a 5 % fractile taking into account the  $\beta$  factor, given in the ETA for the base material under consideration.

Example with 10 tests:

$$N_{Rk1} = N_{Rm} \cdot (1 - 2,57 \cdot v) \cdot \beta \leq N_{Rk,ETA} \quad (B.3.1.b)$$

with:  $N_{Rm}$  = mean value of the ultimate load of the 10 tests  
 $v$  = coefficient of variation of the ultimate load  
 $\beta$  = factor to consider the different influences of the product, given in the ETA

The minimum number of pull-out tests is 5; with 5 tests the following equation has to be used:

Example with 5 tests:

$$N_{Rk1} = N_{Rm} \cdot (1 - 3,4 \cdot v) \cdot \beta \leq N_{Rk,ETA} \quad (B.3.1.c)$$

For shear loads it can be assumed:

if  $V_{Rk,ETA} \geq N_{Rk,ETA}$ :  $V_{Rk1} = N_{Rk1}$   $V_{Rk,c}$  according to C.5.2.2.5 (Annex C)  
 if  $V_{Rk,ETA} < N_{Rk,ETA}$ :  $V_{Rk1} = N_{Rk1} \cdot (V_{Rk,ETA} / N_{Rk,ETA})$   $V_{Rk,c}$  according to C.5.2.2.5 (Annex C)

### B.3.3 Proof-load tests

#### B.3.3.1 Execution of proof-load tests

The load should be progressively increased until the proof load  $N_p$  is achieved.

$$N_p \geq 0,8 \cdot N_{Sd} \cdot \gamma_M \cdot 1/\beta \quad (\text{B.3.2})$$

$N_p$  = load  $N_p$  for the proof load tests

$N_{Sd}$  = design value of action ( $N_{Sk} \cdot \gamma_F$ )

$\gamma_M$  = partial safety factors for the resistance ( $\gamma_M = 2,5$ )

$\beta$  = factor to consider the different influences of the product; given in the ETA

#### B.3.3.2 Evaluation of the results of proof-load tests

If visible movement or displacement of the injection anchors does not occur in all tests under the proof-load, then an estimate for the characteristic resistance  $N_{Rk2}$  may be obtained as follows:

$$N_{Rk2} = 1/0,8 \cdot N_p \cdot \beta \leq N_{Rk,ETA} \quad (\text{B.3.3})$$

$N_{Rk,ETA}$  = characteristic resistance  $F_{Rk}$  given in the ETA for the same category of masonry

$N_p$  = see Equation (B.3.2)

$\beta$  = factor to consider the different influences of the product; given in the ETA

For shear loads it can be assumed:

if  $V_{Rk,ETA} \geq N_{Rk,ETA}$ :  $V_{Rk2} = N_{Rk2}$   $V_{Rk,c}$  according to C.5.2.2.5 (Annex C)

if  $V_{Rk,ETA} < N_{Rk,ETA}$ :  $V_{Rk2} = N_{Rk2} \cdot (V_{Rk,ETA} / N_{Rk,ETA})$   $V_{Rk,c}$  according to C.5.2.2.5 (Annex C)

### B.4. Test report

The test report should include all information necessary to assess the resistance of the tested injection anchor. It should be given to the person responsible for the design of the fastening. The following information is necessary e.g.:

Name of product

Construction work

Building owner

Date and place of tests

Test rig

Type of structure to be fixed

Masonry (type of brick, strength class, all dimensions of bricks and mortar group if possible);

Visual assessment of masonry (flush joints, joint clearance, regularity);

Thickness of plaster layer or intervening layer (e.g. insulation), if existing

Injection anchors

Cutting diameter of hard metal hammer-drill bits

Type of used drill method (hammer drill, impact drill, core drill)

Cleaning process of the drill hole in detail

Results of tests including indication of value  $N_1$  or  $N_p$ ; mode of failure

Tests carried out or supervised by;                      Signature